- 4. (see Chapter Seven). facility is rural. This decision will be documented in the Preliminary Engineering Report like street system), it may be appropriate to use the urban design criteria even though the Therefore, if the area is "urban" in character (e.g., a densely populated area with a gridfor that functional classification (e.g., arterials) in relatively built-up rural areas. for rural roads and highways. The designer may, as an option, use the "suburban" criteria up, but unincorporated, areas. However, there are many "rural" facilities in Indiana which pass through relatively built-Rural Tables. The rural tables do not provide design criteria for sub-categories. In these cases, it may be inappropriate to use the criteria
- S to include the element in the highway cross section. project design. elements included in a table (e.g., sidewalk width) are not automatically warranted in the Cross Section Elements. The values in the tables will only apply after the decision has been made The designer should realize that some of the cross section
- roadway width than the existing facility. See Section 55-4.05 General Department policy is that a 3R project will not be designed with a narrower
- 6. references for greater insight into the design elements. design values for easy use. Manual Section References. However, the designer should review the Manual section These tables are intended to provide a concise listing of
- .7 design tables. parentheses, e.g., (6). The information in the footnotes is critical to the proper use of the Footnotes. The tables include many footnotes, which are identified by a number in
- $\infty$ design against the criteria presented in this Chapter. to the geometric design of 3R projects. However, the designer will evaluate the proposed design exception. The discussion in Section 40-8.0 on design exceptions applies equally asterisk to indicate controlling design criteria which, if not met, require a Level One Controlling Design Criteria. The 3R tables of geometric design criteria provide an

	Design	Element		Manual Section		2-La	ane		Multi	-Lane	
	Design Year Traffi	ic (AADT)		40-2.01	< 400	400 ≤ AADT < 3000	3000 ≤ AADT < 5000	≥ 5000	Undivided	Divided	
ign	Design Forecast Y	⁄ear		55-4.01		20 Yea	ars (1)		20 Years (1)		
Design Controls	*Design Speed (kn	n/h) (2)		55-4.01	Posted Speed Limit			Posted S	peed Limit		
	Access Control			40-5.0	Partial Control / None			Partial Cor	ntrol / None		
	Level of Service			40-2.0	Desirable: B; Minimum: D			Desirable: B;	Minimum: D		
	Travel Lane	*Width		55-4.05	3.6 m	3.6 m	3.6 m	3.6 m	3.6	3 m	
	Traver Earle	Typical Surface Type (3)		Ch. 52		Asphalt /	Concrete		Asphalt /	Concrete	
		*Width Usable		55-4.05	D: 1.8 m M: 0.6 m	D: 2.4 m M: 0.9 m	D: 2.4 m M: 1.8 m	D: 3.3 m M: 2.4 m	Desirable: 3.3 m Minimum: 2.4 m	Rt: D: 3.3 m; M: 2.7 m Lt: D: 1.2 m; M: 1.2 m	
	Shoulder (4)	*Width F	Paved	55-4.05	D: 1.2 m M: 0.0 m	D: 1.8 m M: 0.6 m	D: 1.8 m M: 0.6 m	D: 3.0 m M: 0.6 m	Desirable: 3.0 m Minimum: 2.4 m	Rt: D: 3.0 m; M: 2.4 m Lt: D: 1.2 m; M: 0.9 m	
Jts .	Typical Surface Type (3)			Ch. 52		halt / Concrete /				/ Sealed Aggregate	
n er	*Travel Lane (5)			55-4.05	- 1-	29			·	%	
Cross Section Elements	Cross Slopes	Shoulde		55-4.05	Paved Width ≤ 1.2 m: 2%; Paved Width > 1.2 m: 4% Asphalt / Concrete; 6% Sealed Aggregate				; Paved Width > 1.2 m: 4% 6% Sealed Aggregate		
ecti	Auxillary Lanes  Lane Width Shoulder Width		55-4.05	De	esirable: 3.6 m;	Minimum: 3.3 i	m		Minimum: 3.3 m		
S S			55-4.05	Des: Sa	me as Next to T	ravel Lane; Min	n: 0.6 m	Des: Same as Next to	Travel Lane; Min: 0.6 m		
ros	Median Width		55-4.05		N/	Ά		0.0 m	Existing		
	Obstruction Free 2	Zone		55-5.02	See Section 55-5.02				See Section	on 55-5.02	
			Foreslope	ре		2:1 or Fl	atter (7)		2:1 or F	latter (7)	
	Side Slopes	Cut	Ditch Width	55-4.05		(7	<u>'</u> )		(7)		
	olde clopes		Backslope	00 4.00		2:1 or Fl	atter (7)		2:1 or Flatter (7)		
		Fill			2:1 or Flatter (7)			2:1 or Flatter (7)			
	Median Slopes			55-4.05		N/	Ά		Desirable: 8:1;	Maximum: 4:1	
	New or Reconstructed	*Structu	ral Capacity	Ch. 60				HS-25	(8)		
	Bridge	*Clear R	loadway Width (9)	55-6.03			F	full Paved App	roach Width		
	Existing Bridge to Remain	*Structu	ral Capacity	Ch. 72				HS-2	0		
***	in Place	*Clear R	loadway Width	55-6.02			Trave	elway Plus 0.6	m on Each Side		
3ridges**	*Vertical		Replaced ssing Bridge					5.05 ו	m		
	Clearance (Arterial Under)	Existing Overpa	ssing Bridge (11)	55-6.0				4.30 ו			
	(10)	-	ian Bridge				Ne	w: 5.35 m; Ex			
	Vertical Clearance	e (Arterial C	Over Railroad) (12)	Ch. 69				7.00 ı	m		

D or Des: Desirable; M or Min: Minimum.

### GEOMETRIC DESIGN CRITERIA FOR RURAL ARTERIAL (3R Project)

<sup>\*</sup> Controlling design criteria (see Section 40-8.0). \*\* Selection of the cross section and bridge elements is based on the design year traffic volumes irrespective of the design speed.

	Design Elemen	t	Manual Section					
	Design Speed			80 km/h	100 km/h			
	*Stopping Sight Distance	*Stopping Sight Distance		130 m	185 m			
	DecisionSight Distance	Speed / Path / Direction Change	42-2.0	230 m	270 m	315 m		
	Distance	Stop Maneuver		140 m	140 m 170 m			
Alignment Elements	Passing Sight Distance		42-3.0	Existing	Existing	Existing		
leme	Intersection Sight Distance, -3% to +3% (14)		55-4.06	P: 190 m; SU: 235 m	P: 230 m; SU: 280 m	P: 265 m; SU: 320 m		
i E	*Minimum Radii		55-4.03		See Section 55-4.03			
ше	*Superelevation Rate		55-4.03	See Section 55-4.03				
Aligr	*Horizontal Sight Distar	nce	55-4.03	See Section 55-4.03				
	*Vertical Curvature	Crest	55-4.04		See Section 55-4.04			
	(K-values)	Sag	55-4.04		See Section 55-4.04			
	*Maximum	Level	55-4.04	5%	4.5%	4%		
	Grade (13)	Rolling	33-4.04	6%	5.5%	5%		
	Minimum Grade		44-1.03	Desirable: 0.5%; Minimum 0.0%				

<sup>\*</sup> Controlling design criteria (see Section 40-8.0)

Deviations from controlling design criteria should be addressed in an approved design exception. Also, any operational or maintenance changes, permanent or temporary, exclusive of work-zone traffic control that in fact create substandard conditions such as by re-striping to obtain added lane(s) by reducing existing land widths or shoulders, must be addressed in design exceptions whether or not actual construction or reconstruction is involved.

Design exception requests are required for Level One design criteria for each project type as follows:

- a) Non-exempt federally-funded project on the Interstate system requires FHWA approval.
- b) Exempt federally-funded project on the Interstate system requires Chief, Design Division approval.
- c) Non-federally-funded project on the Interstate system requires Chief, Design Division approval with an information copy sent to FHWA.
- d. Project not on the Interstate system requires Chief, Design Division approval.

## GEOMETRIC DESIGN CRITERIA FOR RURAL ARTERIAL (3R Project)

Table 55-3A (Continued)

### GEOMETRIC DESIGN CRITERIA FOR RURAL ARTERIAL (3R Project)

#### Footnotes to Table 55-3A

- (1) Design Forecast Year. For a partial 3R project, the pavement should be designed for at least a 10-year design life.
- (2) <u>Design Speed</u>. The minimum design speed should equal a) the anticipated posted speed limit after construction or b) the state legal limit of 60 mph on a non-posted multilane divided highway or 55mph on a non-posted two-lane highway.
- (3) Surface Type. The pavement type selection will be determined by the Materials and Tests Division's pavement design engineer or by the local jurisdiction.
- (4) <u>Shoulder</u>. The following will apply:
  - a. On an INDOT facility, the shoulder should be paved to the front face of guardrail. The desirable guardrail offset is 0.6 m from the effective usable shoulder width. In a restrictive situation, the guardrail offset may be 0.3 m from the effective usable shoulder width. See Section 49-5.0 for more information.
  - b. If guardrail is present, the minimum offset from E.T.L. to the front face of guardrail should desirably be equal to the shy line distance, but should not be less than 1.2 m. See Section 49-5.0 for shy line offsets.
  - c. Usable shoulder width is defined as the distance from the edge of the travel lane to the shoulder break point.
- (5) <u>Cross Slope (Travel Lane)</u>. Cross slopes of 1.5% are acceptable on an existing bridge to remain in place.
- (6) <u>Cross Slopes (Shoulder)</u>. Table values are for tangent sections. See Figure 45-1A(1) or Figure 45-1A(2) for more-specific information. See Figure 43-3M or Figure 43-3N for shoulder cross slope on a horizontal curve.
- (7) <u>Side Slopes</u>. Section 55-4.05 provides additional information for side slope criteria.
- (8) <u>Structural Capacity (New or Reconstructed Bridge)</u>. The following will apply:
  - a. Each bridge on a facility with greater than 600 trucks per day should be checked using the Alternate Military loading.
  - b. Each State highway bridge within 25 km of a Toll Road gate must be designed for Toll Road Loading.
  - c. Each bridge on an Extra Heavy Duty Highway must be designed for the Michigan Train truck loading configuration.
  - d. See Chapter Sixty for additional information on the loading configurations.
- (9) <u>Width (New or Reconstructed Bridge)</u>. Width is the minimum for a 3R project. See Section 59-1.0 for additional information on bridge width. On a State highway, the minimum clear roadway width should be 9.4 m.

- (10) <u>Vertical Clearance (Arterial Under)</u>. Table values include an additional 150 mm allowance for a future pavement overlay. Vertical clearances apply from usable edge to usable edge of shoulders.
- (11) <u>Vertical Clearance (Existing Bridge)</u>. See Section 55-6.02 for additional information on minimum allowable vertical clearance.
- (12) <u>Vertical Clearance (Arterial Over Railroad)</u>. See Chapter Sixty-nine for additional information on railroad clearance under a highway.
- (13) <u>Maximum Grade</u>. A grade that is 1% steeper may be used for a one-way downgrade.
- (14) <u>Intersection Sight Distance</u>. For left turn onto a 2-lane road. P = Passenger car; SU = single unit truck. See Figure 46-10G for values for combination trucks.

	Design Ele	ement		Manual Section			2-Lane				
Design Controls	Design Year Traffic (AADT)	l		40-2.01	< 400	400 ≤ AADT < 1000	1000 ≤ AADT < 3000	3000 ≤ AADT < 5000	≥ 5000		
Sont	Design Forecast Year			55-4.01			20 Years (1)				
ign (	*Design Speed (km/h) (2)			55-4.01	Posted Speed Limit						
Des	Access Control			40-5.0		None					
	Level of Service			40-2.0		Des	irable: B; Minimun	n: D			
	Travel Lane	*Width		55-4.05	Des: 3.6 m Des: 3.6 m Des: 3.6 m Alin: 3.0 m Min: 3.3 m Min: 3.3 m Alin: 3.6 m (3) 3.6 m (4)						
		Typical S	surface Type (4)	Ch. 52			Asphalt / Concrete				
		*Width Us	sable	55-4.05	Des: 1.2 m Min: 0.6 m	Des: 1.8 m Min: 0.6 m	Des: 2.4 m Min: 0.9 m	Des: 2.4 m Min: 1.8 m	Des: 3.0 m Min: 1.8 m		
ø	Shoulder (5)		*Width Paved		Des: 0.6 m Min: 0.0 m	Des: 1.2 m Min: 0.0 m	Des: 1.2 m Min: 0.6 m	Des: 1.8 m Min: 0.6 m	Des: 2.4 m Min: 0.6 m		
mut		Typical Surface Type (4)		Ch. 52	Asphalt / Concrete / Sealed Aggregate						
E		*Travel Lane (6)		55-4.05	2% Typical; 3% Maximum						
Cross Section Elemnts	Cross Slope	Shoulder	(7)	55-4.05	Paved V	Paved Width ≤ 1.2 m: 2%-3%;  Paved Width > 1.2 m: 4%-6% Asphalt / Concrete; 6% Sealed Aggregate					
Cross	Auxiliary Lanes	Lane Wid	Lane Width		Des: Same as Travel Lane  Min: 3.0 m  Des: Same as Travel Lane  Min: 3.3 m  Des: Same as Next to Travel Lane; Min: 0.6 m						
	Obstruction Free Zone	Silouluei	vviditi	55-5.02	See Section 55-5.02						
	Obstruction Free Zone		Foreslope	33-3.02			2:1 or Flatter (8)	=			
		Cut	Ditch Width	55-4.05			(8)				
	Side Slopes		Backslope				2:1 or Flatter (8)				
		Fill		55-4.05			2:1 or Flatter (8)				
	New or Reconstructed	*Structura	al Capacity	Ch. 60			HS-25 (9)				
	Bridge		adway Width (10)	55-6.03		Full	Paved Approach W	/idth			
	Existing Bridge		al Capacity	Ch. 72			HS-15				
* *s	to Remain in Place	*Clear Ro	adway Width (11)	55-6.02	6.6 m	6.6 m	7.2 m	8.4 m	8.4 m		
Bridges**	*Vertical Clearance (Collector Under)	*Vertical Clearance New or Replaced Overpassing Bridge (12)		55-6.0			4.45 m				
	(Solicotor Oridor)		Existing Overpassing Bridge (13)				4.30 m				
	Vertical Clearance (Collecto	r Over Railroa	ad) (14)	Ch. 69			7.00 m				

Des: Desirable; Min: Minimum.

\* Controlling design criteria (see Section 40-8.0). \*\* Selection of the cross section and bridge elements is based on the design year traffic volumes irrespective of the design speed.

	Design El	ement	Manual Section			2-Lane			
	Design Speed			60 km/h	70 km/h	80 km/h	90 km/h	100 km/h	
	*Stopping Sight Distance	Desirable	55-4.02	85 m	105 m	130 m	160 m	185 m	
	Decision Ciabt Distance	Speed / Path / Direction Change	40.00	170 m	200 m	230 m	270 m	315 m	
	Decision Sight Distance	Stop Maneuver	42-2.0	95 m	115 m	140 m	170 m	200 m	
	Passing Sight Distance	Passing Sight Distance 42-3.0 Existing Existing						Existing	
ents	Intersection Sight Distance,	30/ to ±30/ (16)	55-4.06	P: 125 m	P: 150 m	P: 190 m	P: 230 m	P: 265 m	
еше	intersection signt distance,	-3% (0 +3% (10)	55-4.00	SU: 160 m	SU: 185 m	SU: 235 m	SU: 280 m	SU: 320 m	
Alignment Elements	*Minimum Radii	55-4.03		(	See Section 55-4.0	3			
me	*Superelevation Rate		55-4.03	See Section 55-4.03					
Aligr	*Horizontal Sight Distance		55-4.03	See Section 55-4.03					
	*Vertical Curvature	Crest	55.4.04	See Section 55-4.04					
	(K-values)	Sag	55-4.04		Ç	See Section 55-4.0	4		
	*Maximum	Level	55.4.04	9%	8%	8%	7.5%	7%	
	Grade (15)	Rolling	55-4.04	10%	9%	9%	8.5%	8%	
	Minimum Grade	44-1.03	Desirable: 0.5%; Minimum: 0.0%						

<sup>\*</sup> Controlling design criteria (see Section 40-8.0).

Deviations from controlling design criteria should be addressed in an approved design exception. Also, any operational or maintenance changes, permanent or temporary, exclusive of work-zone traffic control that in fact create substandard conditions such as by re-striping to obtain added lane(s) by reducing existing lane widths or shoulders, must be addressed in design exceptions whether or not actual construction or reconstruction involved.

Design exception requests for Level One design criteria require Chief, Design Division approval.

## GEOMETRIC DESIGN CRITERIA FOR STATE RURAL COLLECTOR ROAD (3R Project)

Table 55-3B (Continued)

### GEOMETRIC DESIGN CRITERIA FOR STATE RURAL COLLECTOR ROAD (3R Project)

#### **Footnotes to Table 55-3B**

- (1) <u>Design Forecast Year</u>. For a partial 3R project, the pavement should be designed for at least a 10-year design life.
- (2) <u>Design Speed</u>. The minimum design speed should equal a) the anticipated posted speed limit after construction or b) the state legal limit (55 mph) on a non-posted highway.
- (3) <u>Travel Lane (Widths)</u>. A minimum 3.3-m travel lane may be used where truck volumes are less than 200 trucks per day.
- (4) <u>Surface Type.</u> The pavement type selection will be determined by the Materials and Tests Division's pavement design engineer or by the local jurisdiction.
- (5) <u>Shoulder</u>. The following will apply:
  - a. On an INDOT facility, the shoulder should be paved to the front face of guardrail. The desirable guardrail offset is 0.3 m from the effective usable shoulder width. In a restrictive situation, the guardrail offset may be 0.3 m from the effective usable shoulder width. See Section 49-5.0 for more information.
  - b. If guardrail is present, the minimum offset from E.T.L. to the front face of guardrail should desirably be equal to the shy line distance, but not less than 1.2 m. See Section 49-5.0 for shy line offsets.
  - c. Usable shoulder width is defined as the distance from the edge of the travel lane to the shoulder break point.
- (6) <u>Cross Slope (Travel Lane)</u>. Cross slopes of 1.5% are acceptable on an existing bridge to remain in place.
- (7) <u>Cross Slopes (Shoulder)</u>. Table values are for tangent sections. See Figure 45-1A(1) or Figure 45-1A(2) for more-specific information. See Figure 43-3M or Figure 43-3N for shoulder cross slope on a horizontal curve.
- (8) <u>Side Slopes</u>. Section 55-4.05 provides additional information for side slope criteria.
- (9) <u>Structural Capacity (New or Reconstructed Bridge)</u>. The following will apply:
  - a. Each bridge on a facility with greater than 600 trucks per day should be checked using the Alternate Military loading.
  - b. Each State highway bridge within 25 km of a Toll Road gate must be designed for Toll Road Loading.
  - c. Each bridge on an Extra Heavy Duty Highway must be designed for the Michigan Train truck loading configuration.
  - d. See Chapter Sixty for additional information on the loading configurations.
- (10) <u>Width (New or Reconstructed Bridge)</u>. Width is the minimum for a 3R project. See Section 59-1.0 for additional information on bridge width. On a State highway, the minimum clear roadway width should be 9.4 m.

- (11) <u>Width (Existing Bridge to Remain in Place)</u>. Clear width will be at least equal to the approach traveled way width or the table values, whichever is greater.
- (12) <u>Vertical Clearance (Collector Under)</u>. Table values include an additional 150 mm allowance for a future pavement overlay. Vertical clearances apply from usable edge to usable edge of shoulders.
- (13) <u>Vertical Clearance (Existing Bridge)</u>. See Section 55-6.02 for additional information on minimum allowable vertical clearance.
- (14) <u>Vertical Clearance (Collector Over Railroad)</u>. See Chapter Sixty-nine for additional information on railroad clearance under a highway.
- (15) <u>Maximum Grades</u>. For a grade less than 150 m in length (PVT to PVC), the maximum grade may be up to 2% steeper than table value. For a road with AADT < 400, the maximum grade may also be 2% steeper.
- (16) <u>Intersection Sight Distance</u>. For left turn onto a 2-lane road, P = Passenger car; SU = single unit truck. See Figure 46-10G for values for combination trucks.

	Design Ele	ement		Manual Section			2-Lane				
	Design Year Traffic (AAD	T)		40-2.01	< 400	400 ≤ AADT < 1000	1000 ≤ AADT < 3000	3000 ≤ AADT < 5000	≥ 5000		
ign	Design Forecast Year			55-4.01		_	20 Years (2)				
Design Controls	*Design Speed (km/h)			55-4.01		See Section 55-4.01 (3)					
	Access Control			40-5.0		None					
	Level of Service			40-2.0		Desi	rable: B; Minimum	: D			
	Travel Lane	*Width (4	)	55-4.05	Des: 3.0 m Min: 2.7 m (4a)	Des: 3.3 m Min: 3.0 m (4b)	Des: 3.3 m Min: 3.0 m (4b)	Des: 3.6 m Min: 3.3 m	Des: 3.6 m Min: 3.3 m (4c)		
		Typical S	Surface Type	Ch. 52		,	Asphalt / Concrete				
		*Width U	sable	55-4.05	Des: 1.2 m Min: 0.6 m	Des: 1.8 m Min: 0.6 m	Des: 1.8 m Min: 0.9 m	Des: 2.4 m Min: 1.8 m	Des: 3.0 m Min: 2.4 m		
nts	Shoulder (5)	*Width Pa	aved	55-4.05	Des: 0.6 m Min: 0.0 m	Des: 0.6 m Min: 0.0 m	Des: 1.2 m Min: 0.6 m	m Des: 1.8 m Des: 2.4 m Min: 0.6 m Min: 0.6 m			
ille ille		Typical Surface Type		Ch. 52		Asphalt / Aggregate / Earth					
Ele		*Travel L	ane (6)	55-4.05			2%-3%				
ection	Cross Slope	Shoulde	r (7)	55-4.05	Paved Paved	Paved Width > 1.2 m: 4%	l Width ≤ 1.2 m: 2% -6% Asphalt; 6%-8		<mark>% Earth</mark>		
Cross Section Elements	Auxiliary Lanes	Lane Wi	dth	55-4.06	Des: 3.0 m;	Min: 2.7 m	Des: 3.3 m;	Min: 3.0 m	Des: 3.6 m Min: 3.0 m		
O	Lailes	Shoulde	r Width			Des: Same as	Next to Travel Land	n; Min: 3.0 m			
	Obstruction-Free Zone			55-5.02		S	ee Section 55-5.02				
			Foreslope				2:1 or Flatter (8)				
	Side Slopes	Cut	Ditch Width	55-4.05			(8)				
	cide diopes		Backslope				2:1 or Flatter (8)				
		Fill		55-4.05			2:1 or Flatter (8)				
		*Structura	al Capacity	Ch. 60			HS-25(8a)				
	New or Reconstructed Bridge	*Clear Ro	padway Width (9)	55-6.03	Travelway +1.2 m	Travelway +1.8 m	Travelway +1.8 m	Travelway +2.4 m	Full Paved Appr. Width		
*	Existing Bridge	*Structura	al Capacity	Ch. 72			HS-15 (10)				
Jes,	to Remain in Place		padway Width (11)	55-6.02	6.6 m	6.6 m	7.2 m	8.4 m	8.4 m		
Bridges**	*Vertical Clearance	New or F		55-6.0		<u> </u>	4.45 m				
	(Collector Under)	Existing Overpassing Bridge		35-0.0			4.30 m				
	Vertical Clearance (Collection	ctor Over Rai	lroad) (13)	Ch. 69			7.00 m				

Des: Desirable; Min: Minimum.

### GEOMETRIC DESIGN CRITERIA FOR LOCAL AGENCY RURAL COLLECTOR ROAD (1) (3R Project)

<sup>\*</sup> Controlling design criteria (see Section 40-8.0). \*\* Selection of the cross section and bridge elements is based on the design year traffic volumes irrespective of the design speed.

	Design El	ement	Manual Section			2-Lane				
	Design Speed		50 km/h	60 km/h	70 km/h	80 km/h	90 km/h			
	*Stopping Sight Distance	55-4.02	65 m	85 m	105 m	130 m	160 m			
	*Stopping Sight Distance  Decision Sight Distance  Passing Sight Distance  Intersection Sight Distance, -39  *Minimum Radii	Speed / Path / Direction Change	42-2.0	145 m	170 m	200 m	230 m	270 m		
	Decision Signi Distance	Stop Maneuver	42-2.0	70 m	95 m	115 m	140 m	170 m		
	Passing Sight Distance		42-3.0	Existing	Existing	Existing	Existing	Existing		
Alignment Elements	Intersection Sight Distance,	55-4.06	P: 105 m SU: 135 m	P: 125 m SU: 160 m	P: 150 m SU: 185 m	P: 190 m SU: 235 m	P: 230 m SU: 280 m			
t E	*Minimum Radii	55-4.03		;	See Section 55-4.03	3				
ше	*Superelevation Rate		55-4.03	See Section 55-4.03						
Aligr	*Horizontal Sight Distance		55-4.03	See Section 55-4.03						
	*Vertical Curvature	Crest	55-4.04		See Section 55-4.04					
	(K-values)	Sag	55-4.04	See Section 55-4.04						
	*Maximum	Level	55-4.04	9%	9%	8%	8%	7%		
	Grade (14)	Rolling	55-4.0 <del>4</del>	11%	10%	9%	9%	8%		
	Minimum Grade	Minimum Grade			Desirable: 0.5%; Minimum: 0.0%					

<sup>\*</sup> Controlling design criteria (see Section 40-8.0).

Deviations from controlling design criteria should be addressed in an approved design exception. Also, any operational or maintenance changes, permanent or temporary exclusive of work-zone traffic control that in fact create substandard conditions such as by re-striping to obtain added lane(s) by reducing existing lane widths or shoulders, must be addressed in design exceptions whether or not actual construction or reconstruction is involved.

Design exception requests for Level One design criteria require Chief, Design Division approval.

# GEOMETRIC DESIGN CRITERIA FOR LOCAL AGENCY RURAL COLLECTOR ROAD<sup>(1)</sup> (3R Project)

Table 55-3C (Continued)

## GEOMETRIC DESIGN CRITERIA FOR LOCAL AGENCY RURAL COLLECTOR ROAD<sup>(1)</sup> (3R Project)

#### **Footnotes to Table 55-3C**

- (1) Applicability. This table is only applicable to a federal-aid funded project.
- (2) Design Forecast Year. For a partial 3R project, the pavement should be designed for at least a 10-year design life.
- (3) <u>Design Speed</u>. The minimum design speed should equal a) the anticipated posted speed limit after construction or b) the state legal limit (55 mph) on a non-posted highway.
- (4) <u>Travel Lane (Width)</u>. A 3.3-m travel lane width should be used where truck volumes exceed 200 trucks per day. In addition, the following will apply:
  - a. Where  $V \ge 80$  km/h, the minimum width is 3.0 m.
  - b. Where  $V \ge 80$  km/h, the minimum width is 3.3 m.
  - c. Where  $V \ge 80$  km/h, the minimum width is 3.6 m.
- (5) <u>Shoulder Width</u>. The following will apply:
  - a. The desirable guardrail offset is 0.3 m from the effective usable shoulder width. See Section 49-5.0 for more information.
  - b. If guardrail is present, the minimum offset from the E.T.L. to face of guardrail should desirably be equal to the shy line offset distance, but not less than 1.2 m (see Section 49-5.0 for shy line offsets).
  - c. Usable shoulder width is defined as the distance from the edge of the travel lane to the shoulder break point.
- (6) <u>Cross Slope (Travel Lane)</u>. Cross slopes of 1.5% are acceptable on an existing bridge to remain in place.
- (7) <u>Cross Slope (Shoulder)</u>. Table values are for tangent sections. See Figure 45-1A(1) or Figure 45-1A(2) for more-specific information. See Figure 43-3M or Figure 43-3N for shoulder cross slope on a horizontal curve.
- (8) <u>Side Slopes</u>. Section 55-4.05 provides additional information for side slope criteria.
- (8a) <u>Structural Capacity (New or Reconstructed Bridge)</u>. A bridge with design year average daily truck traffic (ADTT) greater than 1,000 should be designed for HS 25 live loads. A bridge with an ADTT less than or equal to 1,000 may be designed for HS 25 or HS 20, whichever the LPA elects.

- (9) <u>Width (New or Reconstructed Bridge</u>). The following will apply:
  - a. Where the approach roadway width (travelway plus shoulders) is surfaced, such surfaced width should be carried across all structures.
  - b. The width of each bridge of more than 30 m in length will be analyzed individually. At a minimum, the roadway width of such a bridge should be the width of travel lanes plus a 0.9-m right shoulder and 0.9-m left shoulder.
  - c. See Section 59-1.0 for more information on bridge width.
- (10) Structural Capacity (Existing Bridge to Remain in Place). If the AADT  $\leq$  50, a HS-10 loading is acceptable.
- (11) Width (Existing Bridge to Remain in Place). Clear width should be at least equal to the approach traveled way width or the table value, whichever is greater. For a bridge of more than 30 m in length, the value in the table does not apply. The acceptability of such a bridge will be assessed individually.
- (12) <u>Vertical Clearance (Collector Under)</u>. Table value includes an additional 150 mm allowance for a future pavement overlay. Vertical clearances apply from usable edge to usable edge of shoulders.
- (13) <u>Vertical Clearance (Collector Over Railroad)</u>. See Chapter Sixty-nine for additional information on railroad clearance under a highway.
- Maximum Grades. For a grades of less than 150 m in length (PVT to PVC), the maximum grade may be 2% steeper than table value. For a road with AADT < 400, the maximum grade may also be 2% steeper.
- (15) <u>Intersection Sight Distance</u>. For left turn onto a 2-lane road, P = Passenger car; SU = single unit truck. See Figure 46-10G for values for combination trucks.

	Design El	ement		Manual Section			2-Lane				
Design Controls	Design Year Traffic (AAD)	Γ)		40-2.01	< 400	400-≤ AADT < 1000	1000-≤ AADT < 3000	3000-≤ AADT < 5000	≥ 5000		
Con	Design Forecast Year			55-4.01	20 Years (2)						
gn (	*Design Speed (km/h)			55-4.01		See Section 55-4.01 (3)					
Desi	Access Control			40-5.0		None					
	Level of Service			40-2.0		Des	irable: B; Minimur	m: D			
	Travel Lane	*Width (4	)	55-4.05	Des: 3.0 m; M	in: 2.7 m (4a)	Des: 3.3 m Min: 3.0 m (4b)	Des: 3.6 m Min: 3.3 m (4c)	Des: 3.6 m Min: 3.3 m (4c)		
		Typical S	Surface Type	Ch. 52		Aspha	alt / Concrete / Agg	regate			
	*Width Usable		55-4.05	Min: 0.6 m	Des: 1.2 m Min: 0.6 m	Des: 1.8 m Min: 0.9 m	Des: 1.8 m Min: 1.2 m	Des: 2.4 m Min: 1.8 m			
* *	Typical Surface Type			Ch. 52		Asp	halt / Aggregate / E	arth	•		
nen	*Travel Lane (6)		55-4.05		2%-3% Aspha	alt / Concrete; 6%-	8% Aggregate				
on Eler	Cross Slope	Shoulder	· (7)	55-4.05	Paved	Paved Width ≤ 1.2 m: 2%-3%; Paved Width > 1.2 m: 4%-6% Asphalt; 6%-8% Aggregate; 8% Earth					
Cross Section Elements**	Auxiliary Lanes	Lane Width		55-4.06		Des: Same As Travel Lane  Min: 2.7 m  Des: Same as Travel Lane  Min: 3.0 m			Lane		
Cros		Shoulde	Shoulder Width		Des: 1.2 m; Min: 0.6 m						
	Obstruction Free Zone			55-5.02			See Section 55-5.0	2			
			Foreslope				2:1 or Flatter (8)				
	Side Slopes	Cut	Ditch Width	55-4.05			(8)				
	Cido Ciopos		Backslope				2:1 or Flatter (8)				
		Fill		55-4.05			2:1 or Flatter (8)				
	New or	*Structur	al Capacity	Ch. 60		_	HS-25(8a)		_		
	Reconstructed Bridge	*Clear Ro	padway Width (9)	55-6.03	Travelway +1.2 m		Travelway +1.8 m	ı	Full Paved Appr. Width		
*	Existing Bridge	*Structur	al Capacity	Ch. 72			HS-15 (10)				
ges,	to Remain in Place	*Clear Ro	oadway Width (11)	55-6.02	6.0 m	6.6 m	7.2 m	8.4 m	8.4 m		
Bridges**	*Vertical Clearance		Replaced sing Bridge (12)				4.45 m				
	(Collector Under) Existing Overpassing Br		sing Bridge	55-6.0			4.30 m				
	Vertical Clearance (Collec	tor Over Rail	road) (13)	Ch. 69			7.00 m				

Des: Desirable; Min: Minimum.

<sup>\*</sup> Controlling design criteria (see Section 40-8.0). \*\* Selection of the cross section and bridge elements is based on the design year traffic volumes irrespective of the design speed.

	Design Element					2-Lane			
	Design Speed			50 km/h	60 km/h	70 km/h	80 km/h	90 km/h	
	*Stopping Sight Dis	tance	55-4.02	65 m	85 m	105 m	130 m	160 m	
	Decision Sight	Speed / Path / Direction Change	42-2.0	145 m	170 m	200 m	230 m	270 m	
	Distance	Stop Maneuver	42-2.0	70 m	95 m	115 m	140 m	170 m	
s,	Passing Sight Dista	ance	42-3.0	Existing	Existing	Existing	Existing	Existing	
Elements	Intersection Sight I	Distance , -3% to +3% (14)	55-4.06	P: 105 m	P: 125 m	P: 150 m	P: 170 m	P: 190 m	
ilen	intersection signit	Distance , -3% to +3% (14)	33-4.00	SU: 135 m	SU: 160 m	SU: 185 m	SU: 235 m	SU: 280 m	
	*Minimum Radii		55-4.03		See Section 55-4.03				
J. J	*Superelevation Ra	te	55-4.03	See Section 55-4.03					
Alignment	*Horizontal Sight D	istance	55-4.03		See Section 55-4.03				
4	*Vertical Curvature	Crest	55-4.04		;	See Section 55-4.0	)4		
	(K-values)	Sag	55-4.04	See Section 55-4.04					
		Level	55-4.04	10%	9%	8.5%	8%	7%	
	*Maximum Grade	Rolling	55-4.04	12%	11%	10.5%	10%	9%	
	Minimum Grade		44-1.03		Desirab	le: 0.5%; Minimu	ım: 0.0%		

<sup>\*</sup> Controlling design criteria (see Section 40-8.0).

Deviations from controlling design criteria should be addressed in an approved design exception. Also, any operational or maintenance changes, permanent or temporary, exclusive of work-zone traffic control that in fact create substandard conditions such as by re-striping to obtain added lane(s) by reducing existing lane widths or shoulders, must be addressed in design exceptions whether or not actual construction or reconstruction is involved.

Design exception requests for Level One design criteria require Chief, Design Division approval.

# GEOMETRIC DESIGN CRITERIA FOR RURAL LOCAL ROAD<sup>(1)</sup> (3R Project)

**Table 55-3D** (Continued)

## GEOMETRIC DESIGN CRITERIA FOR RURAL LOCAL ROAD<sup>(1)</sup> (3R Project)

#### **Footnotes to Table 55-3D**

- (1) <u>Applicability</u>. This table is only applicable to a federal-aid funded project.
- (2) <u>Design Forecast Year</u>. For a partial 3R project, the pavement should be designed for at least a 10-year design life.
- (3) <u>Design Speed</u>. The minimum design speed should equal a) the anticipated posted speed limit after construction or b) the state legal limit (55 mph) on a non-posted highway.
- (4) Travel Lane (Width). A 3.3-m travel lane should be used where truck volumes exceed 200 trucks per day. In addition, the following will apply:
  - a. Where  $V \ge 80$  km/h, the minimum width is 3.0 m.
  - b. Where  $V \ge 80$  km/h, the minimum width is 3.3 m.
  - c. Where  $V \ge 80$  km/h, the minimum width is 3.6 m.
- (5) <u>Shoulder Width</u>. The following will apply:
  - a. The desirable guardrail offset is 0.3 m from the effective usable shoulder width. In a restrictive situation, the guardrail offset may be 0.3 m from the effective usable shoulder width. See Section 49-5.0 for more information.
  - b. If guardrail is present, the minimum offset from E.T.L. to face of guardrail should desirably be equal to the shy line offset distance, but not less than 1.2 m (see Section 49-5.0 for shy line offsets).
  - c. Usable shoulder width is defined as the distance from the edge of the travel lane to the shoulder break point.
- (6) <u>Cross Slope (Travel Lane)</u>. Cross slopes of 1.5% are acceptable on an existing bridge to remain in place.
- (7) <u>Cross Slope (Shoulder)</u>. Table values are for tangent sections. See Figure 45-1A(1) or Figure 45-1A(2) for more-specific information. See Figure 43-3M or Figure 43-3N for shoulder cross slope on a horizontal curve.
- (8) <u>Side Slopes</u>. Section 55-4.05 provides additional information for side slope criteria.
- (8a) <u>Structural Capacity (New or Reconstructed Bridge)</u>. A bridge with design year average daily truck traffic (ADTT) greater than 1,000 should be designed for HS 25 live loads. A bridge with an ADTT less than or equal to 1,000 may be designed for HS 25 or HS 20, whichever the LPA elects.
- (9) Width (New or Reconstructed Bridge). The width of a bridge of more than 30 m in length should be analyzed individually. At a minimum, the roadway width of such a bridge will be the width of travel lanes plus a 0.6-m right shoulder and 0.6-m left shoulder. Where shoulders are paved, it is desirable to provide the full roadway width across the bridge. See Section 59-1.0 for more information on bridge width.
- (10) <u>Structural Capacity (Existing Bridge to Remain in Place)</u>. If the AADT  $\leq$  50, an HS-10 loading is acceptable.

- Width (Existing Bridge to Remain in Place). A minimum clear width that is 0.6 m narrower than that shown in the table may be used on a road with few trucks. The clear roadway width should be at least the same width as the approach travelway. For a one-lane bridge, the width may be 5.4 m. For a bridge of more than 30 m in length, the value in the table do not apply. The acceptability of such a bridge will be assessed individually.
- (12) <u>Vertical Clearance (Local Under)</u>. Table values include an additional 150 mm allowance for a future pavement overlay. Vertical clearances apply from usable edge to usable edge of shoulders.
- (13) <u>Vertical Clearance (Local Over Railroad)</u>. See Chapter Sixty-nine for additional information on railroad clearance under a highway.
- (14) <u>Intersection Sight Distance</u>. For left turn onto a 2-lane road, P = Passenger car; SU = single unit truck. See Figure 46-10G for values for combination trucks.

	Design	Element	Manual		Design Values (By Type of Area)	
	Design	Liement	Section	Suburban	Intermediate	Built-Up
	Design Forecas	t Year	55-4.01	20 Years (1)	20 Years (1)	20 Years (1)
Design Controls	*Design Speed	(km/h) (2)	55-4.01	Posted Speed Limit	Posted Speed Limit	Posted Speed Limit
ssig	Access Control		40-5.0	Partial Control / None	None	None
පී යි	Level of Service	е	40-2.0	Des: B; Min: D	Des: C; Min: D	Des: C; Min: D
	On-Street Park	ing	45-1.0	None	Optional (3)	Optional (3)
	Travel Lane	*Width (4)	55-4.05	Curbed: Des: 3.6 m; Min: 3.3 m Uncurbed: Des: 3.6 m; Min: 3.3 m	Curbed: Des: 3.6 m; Min: 3.3 m Uncurbed: Des: 3.6 m; Min: 3.3 m	Curbed: Des: 3.6 m; Min: 3.0 m
		Typical Surface Type (5)	Ch. 52	Asphalt / Concrete	Asphalt / Concrete	Asphalt / Concrete
	*Curb Offset (6)		55-4.05	Des: 0.6 m; Min: 0.3 m	Des: 0.6 m; Min: 0.3 m	Des: 0.6 m; Min: 0.3 m
	Shoulder	*Paved Width (7)	55-4.05	Curbed, Rt. Des: 3.0 m; Min 0.3 m Curbed, Lt. Des: 1.2 m; Min 0.3 m Uncurbed, Rt.: 3.0 m; Lt.: 1.2 m	Curbed, Rt. Des: 2.4 m; Min 0.3 m Curbed, Lt. Des: 0.9 m; Min 0.6 m Uncurbed, Rt.: 2.4 m; Lt.: 0.9 m	Right: 1.8 m; Left: 0.9 m
		Typical Surface Type (5)	Ch. 52	Asphalt / Concrete	Asphalt / Concrete	Asphalt / Concrete
		*Travel Lane (8)	55-4.05	2% - 3%	2% - 3%	2% - 3%
	Cross Slope	Shoulder (9)	55-4.05	Rt.: 4% - 6%; Lt.: 2% - 3%	Paved Width ≤ 1.2 m: 2%-3%; Paved Width > 1.2m: 4%-6%	Paved Width ≤ 1.2 m: 2%-3%; Paved Width > 1.2m: 4%-6%
		Lane Width		Des: 3.6 m; Min: 3.3 m	Des: 3.6 m; Min: 3.0 m	Des: 3.6 m; Min: 3.0 m
ınts	Auxiliary	Curb Offset	55-4.05	Des: 0.3 m; Min: 0.0 m	Des: 0.3 m; Min: 0.0 m	Des: 0.3 m; Min: 0.0 m
В	Lanes	Shoulder Width		Des: 3.0 m; Min: 0.6 m	Des: 2.4 m; Min: 0.6 m	Des: 1.8 m; Min: 0.6 m
<u> </u>		Typical Surface Type (5)	Ch. 52	Asphalt / Concrete	Asphalt / Concrete	Asphalt / Concrete
Section Elements	TWLTL Lane W	/idth	46-5.0	Des: 4.8 m; Min. 4.2 m	Des: 4.8 m; Min: 3.6 m	Des: 4.2 m; Min: 3.3 m
cţi	Parking Lane V	Vidth	45-1.04	N/A	Des: 3.0 m; Min: 2.4 (10)	Des: 3.0 m; Min: 2.4 m (10)
Še	Median	Depressed		Existing	Existing	N/A
Cross	Width	Raised Island	55-4.05	Des: 4.8 m; Min: 0.6 m	Des: 4.8 m; Min: 0.6 m	Des: 4.8 m; Min: 0.6 m
Ě		Flush / Corrugated		Des: 4.8 m; Min: 0.6 m	Des: 4.8 m; Min: 0.6 m	Des: 4.8 m; Min: 0.6 m
O	Sidewalk Width	(11)	55-4.05	1.2 m with 1.5-m Buffer (Des)	Des: 1.8 m; Min: 1.2 m	Des: 1.8 m; Min: 1.2 m
	Bicycle Lane W	,	51-7.0	Curbed: 1.5 m Uncurbed: Shld. Width +1.2 m	Curbed: 1.5 m Uncurbed: Shld. Width +1.2 m	Curbed: 1.5 m
	Obstruction Fre	ee Zone	55-5.02	See Section 55-5.02	See Section 55-5.02	See Section 55-5.02
	Typical Curbing	Type (where used) (13)	55-4.05	Sloping / Vertical	Sloping / Vertical	Sloping / Vertical
		Foreslope		2:1 or Flatter	2:1 or Flatter (14)	N/A
	Side Slopes	Cut Ditch Width	55-4.05	(14)	(14)	N/A
	(Uncurbed)	Backslope	35-4.05	2:1 or Flatter (14)	2:1 or Flatter (14)	N/A
		Fill		2:1 or Flatter (14)	2:1 or Flatter (14)	N/A
	Side Slopes	Cut (Backslope)	55-4.05	(15)	(15)	(15)
	(Curbed)	Fill	33-4.03	2:1 or Flatter (14)	2:1 or Flatter (14)	2:1 or Flatter (14)
	Median Slopes	(Depressed)	55-4.05	Desirable: 8:1; Maximum: 4:1	Desirable: 8:1; Maximum: 4:1	Desirable: 8:1; Maximum: 4:1

<sup>\*</sup> Controlling design criteria (see Section 40-8.0).

Des: Desirable; Min: Minimum

GEOMETRIC DESIGN CRITERIA FOR MULTI-LANE URBAN ARTERIAL (3R Project)

Table 55-3E

	Design [	lamont	Manual			Desig	n Values (By Type of A	Area)		
	Design E	tiernent	Section	Suburba	an		Intermediate			Built-Up
	New or	*Structural Capacity (16)	Ch. 60	HS-25			HS-25			HS-25
	Reconstructed Bridge	*Clear Roadway Width(17)	55-6.03		Cı		ull Approach Curb-to-C ırbed: Full Approach W		h	
	Existing	*Structural Capacity	Ch. 72	HS-20		HS-20			HS-20	
	Bridge to Re- main in Place	*Clear Roadway Width	55-6.02	Curbed: Fu	ıll Approach Cu	rb-to-Cur	b Width; Uncurbed: T	ravelway	Plus 0.6 m o	n Each Side
Bridges	*Vertical	New or Replaced Overpassing Bridge (18a & 18c)		5.05 m	1		5.05 m (18b)		5.	05 m (18b)
	(Arterial Under)	Existing Overpassing Bridge (19)	55-6.0	4.30 m		4.30 m			4.30 m	
	Onder)	Sign Truss / Pedestrian Bridge (18a & 18c)		New: 5.35 m; Exis	sting: 5.20 m	New:	5.35 m; Existing: 5.2	20 m	New: 5.35	m; Existing: 5.20 m
	Vertical Clearan	ce (Arterial over Railroad) (20)	Ch. 69				7.00 m	•		
	Design Speed			50 km/h	60 km/	h	70 km/h	80	) km/h	90 km/h
	*Stopping Sight I	*Stopping Sight Distance		65 m	85 m		105 m	1	30 m	160 m
	Decision Sight Distance	Speed / Path / Direction Change	42-2.0	U: 195 m SU: 170 m	U: 235 SU: 205		U: 275 m SU: 235 m		315 m : 270 m	U: 360 m SU: 315 m
	Distance	Stop Maneuver		155 m	195 m	1	235 m	2	.80 m	325 m
Jts	Intersection Sigh	nt Distance, -3% to +3% (21)	55-4.06	P: 105 m	P: 125	m	P: 150 m	P:	190 m	P: 230 m
Ш G	intersection sign	it Distance, -3 /6 to +3 /6 (21)	33-4.00	SU: 135 m	SU: 160	m	SU: 185 m	SU	: 235 m	SU: 280 m
Ele	*Minimum Radii		55-4.03		•	•	See Section 55-4.03			•
aut	*Superelevation	Rate	55-4.03				See Section 55-4.03			
Ĕ	*Horizontal Sight	Distance	55-4.03				See Section 55-4.03			
Alignment Elements	*Vertical Curvature	Crest	55-4.04				See Section 55-4.04			
	(K-values)	Sag					See Section 55-4.04			
	*Maximum	Level	55-4.04	10%	9%		8.5%		8%	7%
	Grade	Rolling	33-4.04	11%	10%		9.5%		9%	8%
	Minimum Grade	Minimum Grade		Curbed Des: 0.5%; Curbed Min: 0.3% Uncurbed: 0.0%						

<sup>\*</sup> Controlling design criteria (see Section 40-8.0). SU: Suburban U: Urban See note at bottom of Table 55-3A for approval authority for Level One design exceptions.

# GEOMETRIC DESIGN CRITERIA FOR MULTI-LANE URBAN ARTERIAL (3R Project)

**Table 55-3E** (Continued)

### GEOMETRIC DESIGN CRITERIA FOR MULTI-LANE URBAN ARTERIAL (3R Project)

#### Footnotes to Table 55-3E

- (1) <u>Design Forecast Year.</u> For a partial 3R project, the pavement should be designed for at least a 10-year design life.
- (2) <u>Design Speed.</u> The minimum design speed should equal a) the anticipated posted speed limit after construction, or b) the state legal limit on a non-posted highway. The legal limit is 30 mph, but with an engineering study may be raised to a maximum of 55 mph.
- (3) On-Street Parking. In general, on-street parking is discouraged.
- (4) <u>Travel Lane (Width)</u>. For an arterial on the National Truck Network, the right lane must be 3.6m in width. For a non-National Truck Network route, a minimum 3.3-m travel lane should be used where truck volumes exceed 200 trucks a day. See Section 55-4.05.
- (5) <u>Surface Type.</u> The pavement type selection will be determined by the Materials and Tests Division's pavement design engineer or by the local jurisdiction.
- (6) Curb Offset. Vertical curbs which are either continuous or introduced intermittently may be offset 0.3 m.
- (7) Shoulder Width. The table values apply to paved shoulder width. The following will also apply:
  - a. For an uncurbed section, the shoulder is paved to the face of guardrail. The desirable guardrail offset is 0.6 m from the effective usable shoulder width. See Section 49-5.0 for more information.
  - b. For an uncurbed section, a desirable additional 0.3 m of compacted aggregate will be provided.
  - c. If guardrail is present, the minimum offset from E.T.L. to face of guardrail should desirably be equal to the shy line offset distance, but not less than 1.2 m (see Section 49-5.0 for shy line offsets). In a restrictive situation, the guardrail offset may be 0.3 m from the effective usable shoulder width.
  - d. For a curbed section, the curb offset is included in the paved shoulder width.
- (8) <u>Cross Slope (Travel Lane).</u> Cross slopes of 1.5% are acceptable on an existing bridge to remain in place.
- (9) <u>Cross Slope (Shoulder).</u> Table values are for tangent sections. See Figure 45-1A(1) or Figure 45-1A(2) for more-specific information. See Figure 43-3M or Figure 43-3N for shoulder cross slope on a horizontal curve.
- (10) <u>Parking Lane Width.</u> The following will apply:
  - a. Where the parking lane will be used as a travel lane during peak hours or may be converted to a travel lane in the future, the width should be equal to the travel lane width plus the curb offset width (if present).
  - b. A parking lane for residential usage may be 0.3 m narrower.
  - c. The cross slope for a parking lane is typically 1% steeper than that of the adjacent travel lane.

- (11) <u>Sidewalk Width</u>. Table values are for the installation of new sidewalks. An existing sidewalk width of 0.9 m or greater (with or without a buffer) may be retained. A buffer strip of 1.2 m or more is desirable.
- (12) <u>Bicycle Lane Width</u>. The widths in the table are in addition to the width of a parking lane, if present. See Section 51-7.0 for additional details.
- (13) Curbing Type. Vertical curbs may only be used with design speed lower than 80 km/h.
- (14) Side Slopes. Section 55-4.05 provides additional information for side slope criteria.
- (15) <u>Side Slope (Curbed) Cut</u>. Typically, a shelf or sidewalk will be present immediately behind the curb before the toe of the backslope. The minimum width of a shelf desirably should be 1.8 m. Where a sidewalk is present, the toe of the backslope will typically be 0.3 m beyond the edge of sidewalk. See Section 45-3.0 for more information.
- (16) <u>Structural Capacity (New or Reconstructed Bridge)</u>. The following will apply:
  - a. Each bridge on a facility with greater than 600 trucks per day should be checked using the Alternate Military Loading.
  - b. Each State highway bridge within 25 km of a Toll Road gate must be designed for Toll Road Loading.
  - c. Each bridge on an Extra Heavy Duty Highway must be designed for the Michigan Train truck loading configuration.
  - d. See Chapter Sixty for additional information on the loading criteria.
- (17) Width (New or Reconstructed Bridge). Widths are minimums for a 3R project. See Section 59-1.0 for additional information on bridge width.
- (18) <u>Vertical Clearance (Arterial Under Railroad)</u>. The following will apply:
  - a. Table values include an additional 150-mm allowance for a future pavement overlay.
  - b. In a highly urbanized area, a minimum clearance of 4.30 m may be provided if there is at least one route with a 4.90-m clearance.
  - c. Vertical clearances apply from usable edge to usable edge of shoulder.
- (19) Vertical Clearance (Existing Bridge). See Section 55-6.02 for additional information on minimum allowable vertical clearances.
- (20) <u>Vertical Clearance (Arterial Over Railroad)</u>. See Chapter Sixty-nine for additional information on railroad clearance under a highway.
- (21) <u>Intersection Sight Distance</u>. For left turn onto a 2-lane road, P = Passenger car; SU = single unit truck. See Figure 46-10G for values for combination trucks.

	Danima	Elamant.	Manual		Design Values (By Type of Area)	
	Design	Element	Section	Suburban	Intermediate	Built-up
	Design Forecas	st Year	55-4.01	20 Years (1)	20 Years (1)	20 Years (1)
<u>⊆</u>	*Design Speed	(km/h) (2)	55-4.01	Posted Speed Limit	Posted Speed Limit	Posted Speed Limit
Design Controls	Access Control		40-5.01	Partial Control / None	None	None
ద ద	Level of Service	е	40-2.0	Des: B; Min: D	Des: C; Min: D	Des: C; Min: D
	On-Street Parki	ing	45-1.0	None	Optional (3)	Optional (3)
	Travel Lane	*Width (4)	55-4.05	Curbed: Des: 3.6 m; Min: 3.3 m Uncurbed: Des: 3.6 m; Min: 3.3 m	Curbed: Des: 3.6 m; Min: 3.3 m Uncurbed: Des: 3.6 m; Min: 3.3 m	Curbed Des: 3.6m Curbed Min: 3.0 m
		Typical Surface Type (5)	Ch. 52	Asphalt / Concrete	Asphalt / Concrete	Asphalt / Concrete
	*Curb Offset (6)	•	55-4.05	Des: 0.6 m; Min: 0.3 m	Des: 0.6 m; Min: 0.3 m	Des: 0.6 m; Min: 0.3 m
	Shoulder	*Paved Width (7)	55-4.05	Curbed Des: 3.0 m; Min. 0.3 m Uncurbed: Des: 3.0 m; Min. 1.8 m	Curbed: Des: 2.4 m; Min: 0.3 m Uncurbed: Des: 2.4 m; Min. 1.2 m	Des: 1.8 m; Min: 0.6 m
		Typical Surface Type (5)	Ch. 52	Asphalt / Concrete	Asphalt / Concrete	Asphalt / Concrete
		*Travel Lane (8)	55-4.05	2%-3%	2%-3%	2%-3%
	Cross Slope	Shoulder (9)	55-4.05	4%-6%	Paved Width ≤ 1.2 m: 2%-3%; Paved Width > 1.2m: 4%-6%	Paved Width ≤ 1.2 m: 2%-3%; Paved Width > 1.2m: 4%-6%
ents		Lane Width		Des: 3.6 m; Min: 3.3 m	Des: 3.6 m; Min: 3.3 m	Des: 3.6 m; Min: 3.0 m
i ii	Auxiliary	Curb Offset	55-4.05	Des: 0.3 m; Min: 0.0 m	Des: 0.3 m; Min: 0.0 m	Des: 0.3 m; Min: 0.0 m
<u> </u>	Lanes	Shoulder Width		Des: 3.0 m; Min: 0.6 m	Des: 2.4 m; Min: 0.6 m	Des: 1.8 m; Min: 0.6 m
fjon		Typical Surface Type (5)	Ch. 52	Asphalt / Concrete	Asphalt / Concrete	Asphalt / Concrete
ec	TWLTL Lane W	/idth	46-5.0	Des: 4.8 m; Min. 4.2 m	Des: 4.8 m; Min: 3.6 m	Des: 4.8 m; Min: 3.3 m
SS	Parking Lane V	Vidth	45-1.04	N/A	Des: 3.0 m; Min: 2.4 m (10)	Des: 3.0 m; Min: 2.4 m (10)
Cross Section Elements	Sidewalk Width	(11)	45-1.06	1.2 m with 1.5-m Buffer (Des)	Des: 1.8 m; Min: 1.2 m	Des: 1.8 m; Min: 1.2 m
	Bicycle Lane W	/idth (12)	51-7.0	Curbed: 1.5 m Uncurbed: Shld. Width +1.2 m	Curbed: 1.5 m Uncurbed: Shld. Width +1.2 m	Curbed: 1.5 m
	Obstruction Fre	e Zone	55-5.02	See Section 55-5.02	See Section 55-5.02	See Section 55-5.02
	Typical Curbing	Type (where used) (13)	55-5.0	Sloping / Vertical	Sloping / Vertical	Sloping / Vertical
		Foreslope		2:1 or Flatter (14)	2:1 or Flatter (14)	N/A
	Side Slopes	Cut Ditch Width	55-5.0	(14)	(14)	N/A
	(Uncurbed)	Backslope	33-3.0	2:1 or Flatter (14)	2:1 or Flatter (14)	N/A
		Fill		2:1 or Flatter (14)	2:1 or Flatter (14)	N/A
	Side Slopes	Cut (Backslope)	55-4.05	(15)	(15)	(15)
	(Curbed)	Fill	55-4.05	2:1 or Flatter (14)	2:1 or Flatter (14)	2:1 or Flatter (14)

GEOMETRIC DESIGN CRITERIA FOR TWO-LANE URBAN ARTERIAL (3R Project)

Table 55-3F

Des: Desirable; Min: Minimum.

\* Controlling design criteria (see Section 40-8.0).

	Design E	Ilomont	Manual			Design Values (	By Type of Area)		
	Design	lement	Section	Subu	rban	Interm	ediate	Buil	t-up
	New or	*Structural Capacity (16)	Ch. 60	HS-	· <mark>25</mark>	HS	<mark>-25</mark>	HS	-25
	Reconstructed Bridge	*Clear Roadway Width(17)	55-6.03			urbed: Full Approa Uncurbed: Full Ap			
	Existing	*Structural Capacity	Ch. 72	HS-	·20	HS	-20	HS	-20
	Bridge to Re- main in Place	*Clear Roadway Width	55-6.02	Curbed:	Full Approach Cu	ırb-to-Curb Width;	Uncurbed: Travelw	ay Plus 0.6 m on Ea	ch Side
Bridges	*Vertical Clearance	New or Replaced Overpassing Bridge (18a & 18c)		5.05	5 m	5.05 n	n (18b)	5.05 m	(18b)
	(Arterial Under)	Existing Overpassing Bridge (19)	44-4.0	4.30	) m	4.3	0 m	4.30	) m
	Officery	Sign Truss / Pedestrian Bridge (18a & 18c)		New: 5.35 m; Existing: 5.20 m		New: 5.35 m; Existing: 5.20 m		New: 5.35 m; Existing: 5.20 m	
	Vertical Clearan (20)	ce (Arterial over Railroad)	Ch. 69			7.0	00 m		
	Design Speed			40 km/h	50 km/h	60 km/h	70 k/h	80 km/h	90 km/h
	*Stopping Sight	Distance	55-4.02	50 m	65 m	85 m	105 m	130 m	160 m
	Decision Sight Distance	Speed / Path / Direction Change	42-2.0	U: 160 m SU: 130 m	U: 195 m SU: 170 m	U: 235 m SU: 205 m	U: 275 m SU: 235 m	U: 315 m SU: 270 m	U: 360 m SU: 315 m
	Distance	Stop Maneuver		130 m	155 m	195 m	250 m	270 m	325 m
Alignment Elements	Intersection Sig	ht Distance	55-4.06	P: 85 m SU: 110 m	P: 105 m SU: 135 m	P: 125 m SU: 160 m	P: 150 m SU: 185 m	P: 180 m SU: 235 m	P: 230 m SU: 280 m
:ler	*Minimum Radii		55-4.03			See Secti	on 55-4.03		
E E	*Superelevation		55-4.03				on 55-4.03		
ше	*Horizontal Sigh	t Distance	55-4.03			See Secti	on 55-4.03		
Align	*Vertical Curvature	Crest	55-4.04				on 55-4.04		
	(K-values)	Sag				See Secti	on 55-4.04		
	*Maximum	Level	55-4.04	11%	10%	9%	8.5%	8%	7%
	Grade	Rolling	50 ∓.0-	12%	11%	10%	9.5%	9%	8%
	Minimum Grade	9	44-1.03			Curbed Des: 0.5% Uncurbe	; Curbed Min: 0.39 ed: 0.0%	%	

<sup>\*</sup> Controlling design criteria (see Section 40-8.0). U: Urban; SU: Suburban. Des: Desirable; Min: Minimum. See note at bottom of Table 55-3A for approval authority for Level One design exceptions.

## GEOMETRIC DESIGN CRITERIA FOR TWO-LANE URBAN ARTERIAL (3R Project)

Table 55-3F (Continued)

### GEOMETRIC DESIGN CRITERIA FOR TWO-LANE URBAN ARTERIAL (3R Project)

#### Footnotes to Table 55-3F

- (1) Design Forecast Year. For a partial 3R project, the pavement should be designed for at least a 10-year design life.
- (2) <u>Design Speed</u>. The minimum design speed should equal a) the anticipated posted speed limit after construction, or b) the state legal limit on a non-posted highway. The legal limit is 30 mph, but with an engineering study may be raised to a maximum of 55 mph.
- (3) On-Street Parking. In general, on-street parking is discouraged.
- (4) <u>Travel Lane (Width)</u>. For an arterial on the National Truck Network, the right lane must be 3.6-m in width. For a non-National Truck Network route, a minimum 3.3-m travel lane should be used where truck volumes exceed 200 trucks a day. See Section 55-4.05.
- (5) <u>Surface Type</u>. The pavement type selection will be determined by the Materials and Tests Division's pavement design engineer or by the local jurisdiction.
- (6) Curb Offset. The curb offset should be 0.6 m. Vertical curbs which are either continuous or introduced intermittently may be offset 0.3 m.
- (7) <u>Shoulder Width</u>. The table values apply to paved shoulder widths. The following will also apply:
  - a. For an uncurbed section, the shoulder is paved to the face of guardrail. The desirable guardrail offset is 0.6 m from the effective usable shoulder width. See Section 49-5.0 for more information.
  - b. For an uncurbed section, a desirable additional 0.3 m of compacted aggregate will be provided.
  - c. If guardrail is present, the minimum offset from E.T.L. to face of guardrail should desirably be equal to the shy line offset distance, but not less than 1.2 m (see Section 49-5.0 for shy line offsets). In a restrictive situation, the guardrail offset may be 0.3 m from the effective usable shoulder width.
  - d. For a curbed section, the curb offset is included in the paved shoulder width.
- (8) <u>Cross Slope (Travel Lane).</u> Cross slopes of 1.5% are acceptable on an existing bridge to remain in place.
- (9) <u>Cross Slope (Shoulder).</u> Table values are for tangent sections. See Figure 45-1A(1) or Figure 45-1A(2) for more-specific information. See Figure 43-3M or Figure 43-3N for shoulder cross slope on a horizontal curve.
- (10) <u>Parking Lane Width.</u> The following will apply:
  - a. Where the parking lane will be used as a travel lane during peak hours or may be converted to a travel lane in the future, the width should be equal to the travel lane width plus the curb offset width (if present).
  - b. A parking lane for residential usage may be 0.3 m narrower.
  - c. The cross slope for a parking lane is typically 1% steeper than that for the adjacent travel lane. Buffered strips of 1.2 m or more are desirable.

- (11) <u>Sidewalk Width.</u> Table values are for the installation of new sidewalks. An existing sidewalk width of 0.9 m or greater (with or without a buffer) may be retained. A buffer strip of 1.2 m or wider is desirable.
- (12) <u>Bicycle Lane Width.</u> The widths in the table are in addition to the width of parking lane, if present. See Section 51-7.0 for additional details.
- (13) <u>Curbing Types.</u> Vertical curbs may only be used with design speed lower than 80 km/h.
- (14) <u>Side Slopes.</u> Section 55-4.05 provides additional information for side slope criteria.
- (15) <u>Side Slopes (Curbed) Cut.</u> Typically, a shelf or sidewalk will be present immediately behind the curb before the toe of the backslope. The minimum width of a shelf desirably should be 1.8 m. Where a sidewalk is present, the toe of the backslope will typically be 0.3 m beyond the edge of sidewalk. See Section 45-3.0 for more information.
- (16) <u>Structural Capacity (New or Reconstructed Bridge)</u>. The following will apply:
  - a. Each bridge on a facility with greater than 600 trucks per day should be checked using the Alternate Military Loading.
  - b. Each State highway bridge within 25 km of a Toll Road gate must be designed for Toll Road Loading.
  - c. Each bridge on an Extra Heavy Duty Highway must be designed for the Michigan Train truck loading configuration.
  - d. See Chapter Sixty for additional information on the loading criteria.
- (17) Width (New or Reconstructed Bridge). Widths are minimums for a 3R project. See Section 59-1.0 for additional information on bridge width.
- (18) <u>Vertical Clearance (Arterial Under Railroad)</u>. The following will apply:
  - a. Table value includes an additional 150 mm allowance for a future pavement overlay.
  - b. In a highly urbanized area, a minimum clearance of 4.30 m may be provided if there is at least one route with a 4.90-m clearance.
  - c. Vertical clearances apply from usable edge to usable edge of shoulder.
- (19) <u>Vertical Clearance (Existing Bridge)</u>. See Section 55-6.02 for additional information on minimum allowable vertical clearance.
- (20) <u>Vertical Clearance (Arterial Over Railroad)</u>. See Chapter Sixty-nine for additional information on railroad clearance under a highway.
- (21) <u>Intersection Sight Distance</u>. For left turn onto a 2-lane road. P = Passenger car; SU = single unit truck. See Figure 46-10G for values for combination trucks.

Design Element			Manual		Design Values (By Type of Area)	
			Section	Suburban	Intermediate	Built-Up
n sls	Design Forecast Year		55-4.01	20 Years (1) 20 Years (1)		20 Years (1)
	*Design Speed (km/h) (2)		55-4.01	Posted Speed Limit	Posted Speed Limit	Posted Speed Limit
Design Controls	Access Control		40-5.0	None	None	None
ద్ద స్ర	Level of Service		40-2.0	Desirable: C; Minimum: D	Desirable: C; Minimum: D	Desirable: C; Minimum: D
	On-Street Parking		45-1.0	Optional (3)	Optional (3)	Optional (3)
	Travel Lane	*Width (4)	55-4.05	Curbed: Des: 3.6 m; Min: 3.0 m Uncurbed: Des: 3.6 m; Min: 3.0 m	Curbed: Des: 3.6 m; Min: 3.0 m Uncurbed: Des: 3.6 m; Min: 3.0 m	Curbed Des: 3.6 m Curbed Min: 3.0 m
		Typical Surface Type (5)	Ch. 52	Asphalt / Concrete	Asphalt / Concrete	Asphalt / Concrete
	Curb Offset (6)		55-4.05	Des: 0.6 m; Min: 0.3 m	Des: 0.6 m; Min: 0.3 m	Des: 0.6 m; Min: 0.3 m
	Shoulder	*Paved Width (7)	55-4.05	Curbed Des: 2.4 m; Min. 0.3 m Uncurbed: Des: 2.4 m; Min. 1.2 m	Curbed Des: 1.8 m; Min. 0.3 m Uncurbed: Des: 1.8 m; Min. 0.9 m	Des: 1.2 m; Min: 0.6 m
		Typical Surface Type (5)	Ch. 52	Asphalt / Concrete	Asphalt / Concrete	Asphalt / Concrete
		*Travel Lane (8)	55-4.05	2%-3%	2%-3%	2%-3%
	Cross Slope	Shoulder (9)	55-4.05	Paved Width ≤ 1.2 m: 2%-3%; Paved Width > 1.2m: 4%-6%	Paved Width ≤ 1.2 m: 2%-3%; Paved Width > 1.2m: 4%-6%	Paved Width ≤ 1.2 m: 2%-3%; Paved Width > 1.2m: 4%-6%
	Auxiliary Lanes	Lane Width		Des: 3.6 m; Min: 3.0 m	Des: 3.6 m; Min: 3.0 m	Des: 3.6 m; Min: 2.7 m
ents		Curb Offset	55-4.05	Des: 0.3 m; Min: 0.0 m	Des: 0.3 m; Min: 0.0 m	Des: 0.3 m; Min: 0.0 m
ы		Shoulder Width		Des: 2.4 m; Min: 0.6 m	Des: 1.8 m; Min: 0.6 m	Des: 1.2 m; Min: 0.6 m
Cross Section Elements		Typical Surface Type (5)	Ch. 52	Asphalt / Concrete	Asphalt / Concrete	Asphalt / Concrete
tion	TWLTL Lane Width		46-5.0	Des: 4.8 m; Min: 3.6 m	Des: 4.2 m; Min: 3.3 m	Des: 4.2 m; Min: 3.0 m
Sec	Parking Lane Width		45-1.04	Des: 3.0 m; Min: 2.4 m	Des: 3.0 m; Min: 2.4 m (10)	Des: 3.0 m; Min: 2.4 m (10)
SS	Median Width	Raised Island	55-4.05	Des: 4.8 m; Min: 0.6 m	Des: 4.8 m; Min: 0.6 m	Des: 4.8 m; Min: 0.6 m
ğ		Flush / Corrugated	00 4.00	Des: 4.8 m; Min: 0.6 m	Des: 4.8 m; Min: 0.6 m	Des: 4.8 m; Min: 0.6 m
	Sidewalk Width (11)		55-4.05	1.2 m with 1.5-m Buffer (Des)	Des: 1.8 m; Min: 1.2 m	Des: 1.8 m; Min: 1.2 m
	Bicycle Lane Width (12)		51-7.0	Curbed: 1.5 m Uncurbed: Shld. Width +1.2 m	Curbed: 1.5 m Uncurbed: Shld. Width +1.2 m	Curbed: 1.5 m
	Obstruction Free Zone		55-5.02	See Section 55-5.02	See Section 55-5.02	See Section 55-5.02
	Typical Curbin	Typical Curbing Type (where used) (13)		Sloping / Vertical	Sloping / Vertical	Sloping / Vertical
	Side Slopes (Uncurbed)	Foreslope		2:1 or Flatter (14)	2:1 or Flatter (14)	N/A
		Cut Ditch Width	55-4.05	(14)	(14)	N/A
		Backslope	55-4.05	2:1 or Flatter (14)	2:1 or Flatter (14)	N/A
		Fill		2:1 or Flatter (14)	2:1 or Flatter (14)	N/A
	Side Slopes	Cut (Backslope)	55-4.05	(15)	(15)	(15)
	(Curbed)	Fill	3000	2:1 or Flatter (14)	2:1 or Flatter (14)	2:1 or Flatter (14)

Des: Desirable; Min: Minimum.
\* Controlling design criteria (see Section 40-8.0).

Design Element			Manual						
			Section	Subu	rban	Intermediate		Built-Up	
	New or	*Structural Capacity (16)	Ch. 60	HS-25		HS-25		HS-25	
	Reconstructed Bridge	*Clear Roadway Width(17)	55-6.03	Curbed: Full Approach Curb-to-Curb Uncurbed: Full Approach Paved W					
	_	*Structural Capacity	Ch. 72	HS-15		HS-15		HS-15	
Bridges	Existing Bridge to Re- main in Place	*Clear Roadway Width	55-6.02	Curbed: Full Approach Curb-to-Curb Width Uncurbed: Travelway Plus 0.6 m on Each Side			Curbed: Full Approach Curb- to-Curb Width Uncurbed: Travelway + 0.3 m on Each Side		
	*Vertical Clearance			4.45 m 4.		4.45	5 m 4.45 m		15 m
	(Collector)	Existing Overpassing Bridge (19)	55-6.0	4.30 m		4.30 m		4.30 m	
	Vertical Clearance (Collector over Railroad) (20)		Ch. 69	7.00 m					
	Design Speed			40 km/h	50 km/h	60 km/h	70 km/h	80 km/h	90 km/h
	*Stopping Sight Distance		55-4.02	50 m	65 m	85 m	105 m	130 m	160 m
	Decision Sight Distance	Speed / Path / Direction Change	42-2.0	U: 160 m SU: 130 m	U: 195 m SU: 170 m	U: 235 m SU: 205 m	U: 275 m SU: 235 m	U: 315 m SU: 270 m	U: 360 m SU: 315 m
	Distance	Stop Maneuver		130 m	155 m	195 m	235 m	280 m	325 m
Alignment Elements	Intersection Sight Distance, -3% to +3% (22)		55-4.06	P: 85 m SU: 110 m	P: 105 m SU: 135 m	P: 125 m SU: 160 m	P: 150 m SU: 185 m	P: 190 m SU: 235 m	P: 230 m SU: 280 m
leπ	*Minimum Radii		55-4.03	See Section 55-4.05					
¥	*Superelevation Rate (24)		55-4.03	See Section 55-4.05					
ше	*Horizontal Sight Distance		55-4.03	See Section 55-4.05					
Align	*Vertical Curvature	Clesi		See Section 55-4.04					
	(K-values)	Sag	55-4.04	See Section 55-4.04					
	*Maximum Grade (21)	Level	55-4.04	11%	11%	11%	10%	9%	8%
		Rolling		14%	13%	12%	11%	10%	9%
	Minimum Grade		44-1.03	Curbed Des: 0.5%; Curbed Min: 0.3% Uncurbed: 0.0%					

### GEOMETRIC DESIGN CRITERIA FOR URBAN COLLECTOR (3R Project)

**Table 55-3G** (Continued)

Controlling design criteria (see Section 40-8.0). SU: Suburban; U: Urban. Des: Desirable; Min: Minimum. For a state-route project, see note at bottom of Table 55-3B for approval authority for Level One design exceptions. For a federally-funded local project, see note at bottom of Table 55-3C for approval authority for Level One design exceptions.

## GEOMETRIC DESIGN CRITERIA FOR URBAN COLLECTOR (3R Project)

#### Footnotes to Table 55-3G

- (1) <u>Design Forecast Year</u>. For a partial 3R project, the pavement should be designed for at least a 10-year design life.
- (2) <u>Design Speed</u>. The minimum design speed should equal a) the anticipated posted speed limit after construction, or b) the state legal limit on a non-posted highway. The legal limit is 30 mph, but with an engineering study may be raised to a maximum of 55 mph.
- (3) <u>On-Street Parking</u>. In general, on-street parking is discouraged.
- (4) <u>Travel Lane (Width)</u>. A minimum 3.3-m travel lane should be used where truck volumes exceed 200 trucks per day. See Section 55-4.05.
- (5) <u>Surface Type</u>. The pavement type selection will be determined by the Materials and Tests Division's pavement design engineer or by the local jurisdiction.
- (6) Curb Offset. The curb offset should be 0.6 m. Vertical curbs which are either continuous or introduced intermittently should be offset 0.3 m.
- (7) <u>Shoulder Width</u>. The table values apply to paved shoulder widths. The following will also apply:
  - a. For an uncurbed section, the shoulder is paved to the face of guardrail. The desirable guardrail offset is 0.6 m from the effective usable shoulder width. See Section 49-5.0 for more information.
  - b. For an uncurbed section, a desirable additional 0.3 m of compacted aggregate will be provided.
  - c. If guardrail is present, the minimum offset from the E.T.L. to face of guardrail should desirably be equal to the shy line offset distance, but not less than 1.2 m (see Section 49-5.0 for shy line offsets). In a restrictive situation, the guardrail offset may be 0.3 m from the effective usable shoulder width.
  - d. For a curbed section, the curb offset is included in the paved shoulder width.
- (8) <u>Cross Slope (Travel Lane)</u>. Cross slopes of 1.5% are acceptable on an existing bridge to remain in place.
- (9) <u>Cross Slope (Shoulder).</u> Table values are for tangent sections. See Figure 45-1A(1) or Figure 45-1A(2) for more-specific information. See Figure 43-3M or Figure 43-3N for shoulder cross slope on a horizontal curve.
- Parking Lane Width. A parking lane for residential usage may be 0.3 m less. The cross slope for a parking lane is typically 1% steeper than that for the adjacent travel lane. In a residential area, a parallel parking lane from 2.1 to 2.4 m in width should be provided on one or both sides of the street. In a commercial or industrial area, the parking lane width should range from 2.4 to 3.3 m, and should usually be provided on both sides of the street. Where curband-gutter sections are used, the gutter pan width may be considered as part of the parking lane width. Where practical, the parking lane width should be in addition to the gutter pan width.
- (11) <u>Sidewalk Width</u>. Table values are for the installation of new sidewalks. An existing sidewalk width of 0.9 m or greater (with or without a buffer) may be retained. A buffer strip of 1.2 m or wider is more desirable.
- (12) <u>Bicycle Lane Width</u>. The widths in the table are in addition to the width of parking lane, if present. See Section 51-7.0 for additional details.

- (13) <u>Curbing Type</u>. Vertical curbs may only be used with design speed lower than 80 km/h.
- (14) <u>Side Slopes</u>. Section 55-4.05 provides additional information for side slope criteria.
- (15) <u>Side Slope (Curbed) Cut</u>. Typically, a shelf or sidewalk will be present immediately behind the curb before the toe of the backslope. The minimum width of a shelf desirably should be 1.8 m. Where a sidewalk is present, the toe of the backslope will typically be 0.3 m beyond the edge of sidewalk. See Section 45-3.0 for more information.
- (16) <u>Structural Capacity (New or Reconstructed Bridge)</u>. The following will apply:
  - a. Each bridge on a facility with greater than 600 trucks per day should be checked using the Alternate Military Loading.
  - b. Each State highway bridge within 25 km of a Toll Road gate must be designed for Toll Road Loading.
  - c. Each bridge on an Extra Heavy Duty Highway must be designed for the Michigan Train truck loading configuration.
  - d. See Chapter Sixty for additional information on the loading criteria.
- (17) Width (New or Reconstructed Bridge). Widths are minimums for a 3R project. See Section 59-1.0 for additional information on bridge width.
- (18) <u>Vertical Clearance (Collector Under Railroad)</u>. Table value includes an additional 150 mm allowance for a future pavement overlay. Vertical clearances apply from usable edge to usable edge of shoulder.
- (19) <u>Vertical Clearance (Existing Bridge)</u>. See Section 55-6.02 for additional information on minimum allowable vertical clearance.
- (20) <u>Vertical Clearance (Arterial Over Railroad)</u>. See Chapter Sixty-nine for additional information on railroad clearance under a highway.
- Maximum Grades. For a grade of less than 150 m in length (PVT to PVC), a one-way downgrade, or a street with AADT < 400, the maximum grade may be 2% steeper than table value. Where adjacent sidewalks are present, the maximum desirable grade is 5%.
- (22) <u>Intersection Sight Distance</u>. For left turn onto a 2-lane road, P = Passenger car; SU = single unit truck. See Figure 46-10G for values for combination trucks.

Design Element			Manual	Manual Design Values (By Type of Area)					
Design Element			Section	Suburban	Intermediate	Built-Up			
	Design Forecast Year		55-4.01	20 Years (1)	20 Years (1) 20 Years (1)				
Design Controls	*Design Speed (km/h) (2)		55-4.01	See Section 55-4.01	See Section 55-4.01	See Section 55-4.01			
ssig	Access Control		40-5.0	None	None	None			
ద్ద ర్ర	Level of Service		40-2.0	Desirable: C; Minimum: D	Desirable: C; Minimum: D	Desirable: C; Minimum: D			
	On-Street Parking		45-1.0	Optional (3)	Optional	Optional			
	Travel Lane	*Width (4)	55-4.05	Curbed: Des: 3.3 m; Min: 3.0 m Uncurbed: Des: 3.3 m; Min: 3.0 m	Curbed: Des: 3.0 m; Min: 2.7 m Uncurbed: Des: 3.3 m; Min: 3.0 m	Curbed Des: 3.0 m Curbed Min: 2.7 m			
		Typical Surface Type	Ch. 52	Asphalt / Concrete Asphalt / Concrete		Asphalt / Concrete			
	*Curb Offset (5)		55-4.05	Des: 0.6 m; Min: 0.3 m	Des: 0.6 m; Min: 0.3 m	Des: 0.6 m; Min: 0.3 m			
	Shoulder	*Usable Width	55-4.05	Curbed Des: 1.2 m; Min. 0.3 m Uncurbed: Des: 1.2 m; Min. 0.6 m	Curbed Des: 1.2 m; Min. 0.3 m Uncurbed: Des: 1.2 m; Min. 0.6 m	Des: 1.2 m; Min: 0.6 m			
		Typical Surface Type	Ch. 52	Asphalt / Concrete / Aggregate / Earth	Asphalt / Concrete / Aggregate / Earth	Asphalt / Concrete / Aggregate / Earth			
	Cross Slope	*Travel Lane (6)	55-4.05	2%-3%	2%-3%	2%-3%			
unts		Shoulder (7)	55-4.05	2%-3% Asphalt / Concrete; 6%-8% Aggregate; 8% Earth	2%-3% Asphalt / Concrete; 6%-8% Aggregate; 8% Earth	2%-3% asphalt / Concrete; 6%-8% Aggregate; 8% Earth			
E E	Auxiliary Lanes	Lane Width		Des: 3.3 m; Min: 3.0 m	Des: 3.3 m; Min: 2.7 m	Des: 3.0 m; Min: 2.7 m			
H H		Curb Offset	55-4.05	Des: 0.3 m; Min: 0.0 m	Des: 0.3 m; Min: 0.0 m	Des: 0.3 m; Min: 0.0 m			
- u		Shoulder Width		Des: 1.2 m; Min: 0.3 m	Des: 1.2 m; Min: 0.3 m	Des: 1.2 m; Min: 0.3 m			
Section Elements		Typical Surface Type	Ch. 52	Asphalt / Concrete / Aggregate / Earth	Asphalt / Concrete / Aggregate / Earth	Asphalt / Concrete / Aggregate / Earth			
	Parking Lane Width (3)		45-1.04	Des: 2.7 m; Min: 2.1 m	Des: 2.7 m; Min: 2.1 m	Des: 2.7 m; Min: 2.1 m			
Cross	Sidewalk Width (8)		55-4.05	1.2 m with 1.5-m Buffer (Des)	Des: 1.8 m; Min: 1.2 m	Des: 1.8 m; Min: 1.2 m			
	Bicycle Lane Width (9)		51-7.0	Curbed: 1.5 m Uncurbed: Shld. Width +1.2 m	Curbed: 1.5 m Uncurbed: Shld. Width +1.2 m	Curbed: 1.5 m			
	Obstruction Free Zone		55-5.02	See Section 55-5.02	See Section 55-5.02	See Section 55-5.02			
	Typical Curbing Type (where used) (5)		55-4.05	Sloping / Vertical Sloping / Vertical		Sloping / Vertical			
	Side Slopes (Uncurbed)	Foreslope	_	2:1 or Flatter (10)	2:1 or Flatter (10)	N/A			
		Cut Ditch Width	55-4.05	(10)	(10)	N/A			
		Backslope	00 4.00	2:1 or Flatter (10)	2:1 or Flatter (10)	N/A			
		Fill		2:1 or Flatter (10)	2:1 or Flatter (10)	N/A			
	Side Slopes			(11)	(11)	(11)			
	(Curbed) Fill		55-4.05	2:1 or Flatter (10)	2:1 of Flatter (10)	2:1 or Flatter (10)			

### GEOMETRIC DESIGN CRITERIA FOR URBAN LOCAL STREET (3R Project)

**Table 55-3H** 

Des: Desirable; Min: Minimum.

\* Controlling design criteria (see Section 40-8.0).

\*\* Table applies only to a project with federal-aid funds.

Design Element			Manual Design Values (By Type of Area)						
			Section	Suburban		Intermediate		Built-Up	
	New or *Structural Capacity		Ch. 60	HS-25(11a)		HS-25(11a)		HS-25(11a)	
	Reconstructed *Clear Roadway Width Bridge		55-6.03	Curbed: Full Approach Curb-to-Curb Width Uncurbed: (12)					
	Existing	*Structural Capacity (13)	Ch. 72	HS-15		HS-15		HS-15	
Bridges	Bridge to Re- main in Place	*Clear Roadway Width	55-6.02	Existing Width (14)					
卤	*Vertical Clearance	New or Replaced Overpassing Bridge (15)	44-4.0	4.45 m		4.45 m		4.45 m	
	(Local Under)	Existing Overpassing Bridge (16)	11 1.0	4.30 m		4.30 m		4.30 m	
	Vertical Clearance (Local over Railroad) (17)		Ch. 69	7.00 m					
	Design Speed			40 km/h	50 km/h	60 km/h	70 km/h	80 km/h	90 km/h
	*Stopping Sight I	Distance	55-4.02	50 m	65 m	85 m	105 m	130 m	160 m
	Decision Sight Distance	Speed / Path / Direction Change	42-2.0	U: 160 m SU: 130 m	U: 195 m SU: 170 m	U: 235 m SU: 205 m	U: 275 m SU: 235 m	U: 315 m SU: 270 m	U: 360 m SU: 315 m
		Stop Maneuver		130 m	155 m	195 m	235 m	280 m	325 m
Alignment Elements	Intersection Sight Distance, -3% to +3% (18)		55-4.06	P: 85 m SU: 110 m	P: 105 m SU: 135 m	P: 125 m SU: 160 m	P: 150 m SU: 185 m	P: 170 m SU: 235 m	P: 190 m SU: 280 m
iii e	*Minimum Radii	55-4.03	See Section 55-4.03						
t T	*Superelevation Rate			See Section 55-4.03					
a me	*Horizontal Sight Distance		55-4.03	See Section 55-4.03					
Aligr	*Vertical Curvature (K-values)	Crest	55-4.04	See Section 55-4.04					
		Sag	33-4.04	See Section 55-4.04					
	*Maximum Grade	Level Rolling	55-4.04	In a residential area, the maximum grade should not exceed 15%. In an industrial or commercial area, the maximum grade should not exceed 8%.					
	Minimum Grade		55-4.04	Curbed Des: 0.5%; Curbed Min: 0.3% Uncurbed: 0.0%					

U: Urban; SU: Suburban. Des: Desirable; Min: Minimum.

\* Controlling design criteria (see Section 40-8.0).

\*\* Table applies only to a project with federal-aid funds.

See note at bottom of Table 55-3D for approval authority for Level One design exceptions.

### GEOMETRIC DESIGN CRITERIA FOR URBAN LOCAL STREET \*\* (3R Project)

Table 55-3H (Continued)

### GEOMETRIC DESIGN CRITERIA FOR URBAN LOCAL STREET (3R Project)

#### Footnotes to Table 55-3H

- (1) Design Forecast Year. For a partial 3R project, the pavement should be designed for at least a 10-year design life.
- (2) <u>Design Speed</u>. The minimum design speed should equal a) the anticipated posted speed limit after construction, or b) the state legal limit on a non-posted highway. The legal limit is 30 mph, but with an engineering study may be raised to a maximum of 55 mph.
- On-Street Parking. In general, on-street parking is discouraged. However, if parking lanes are used, cross slopes are typically 1% steeper than that of the adjacent travel lane. In a residential area, a parallel parking lane from 2.1 to 2.4 m in width should be provided on one or both sides of the street. In a commercial or industrial area, parking lane width should range from 2.4 to 3.3 m, and should usually be provided on both sides of the street. Where curb-and-gutter sections are used, the gutter pan width may be considered as part of the parking lane width. Where practical, the parking lane width should be in addition to the gutter pan width.
- (4) <u>Travel Lane (Width)</u>. A minimum 3.3-m travel lane should be used where truck volumes exceed 200 trucks per day. See Section 55-4.05.
- (5) <u>Curb Offset</u>. A vertical-curb offset should be 0.6 m. Vertical curbs which are either continuous or introduced intermittently may be offset 0.3 m. A sloping-curb offset may be zero. For a curbed section, the curb offset is included in the paved shoulder width. Vertical curbs may only be used with design speed lower than 80 km/h.
- (6) <u>Cross Slope (Travel Lane)</u>. Cross slopes of 1.5% are acceptable on an existing bridge to remain in place.
- (7) <u>Cross Slope (Shoulder)</u>. Table values are for tangent sections; see Section 43-3.06 for shoulder cross slopes on a horizontal curve.
- (8) <u>Sidewalk Width</u>. Table values are for the installation of new sidewalks. An existing sidewalk width of 0.9 m or greater (with or without a buffer) may be retained. A buffer strip of 1.2 m or wider is desirable.
- (9) <u>Bicycle Lane Width</u>. The widths in the table are in addition to the width of parking lane, if present. See Section 51-7.0 for additional details.
- (10) <u>Side Slopes</u>. Section 55-4.05 provides additional information for side slope criteria.
- (11) Side Slope (Curbed) Cut. Typically, a shelf or sidewalk will be present immediately behind the curb before the toe of the backslope. The minimum width of a shelf desirably should be 1.8 m. Where a sidewalk is present, the toe of the backslope will typically be 0.3 m beyond the edge of sidewalk. See Section 45-3.0 for more information.
- (11a) <u>Structural Capacity (New or Reconstructed Bridge)</u>. A bridge with design year average daily truck traffic (ADTT) greater than 1,000 should be designed for HS 25 live loads. A bridge with an ADTT less than or equal to 1,000 may be designed for HS 25 or HS 20, whichever the LPA elects.

(12) <u>Width (New or Reconstructed Bridge) Uncurbed</u>. The following will apply:

 $\frac{\text{Volume}}{0 < \text{AADT} < 400}$   $400 \le \text{AADT} < 5000$   $AADT \ge 5000$ 

Minimum Clear Width
Travelway + 0.6 m each side
Travelway + 0.9 m each side
Approach Roadway Width (Travelway Plus Shoulders)

See Section 59-1.0 for more information on bridge width.

- (13) <u>Structural Capacity (Existing Bridge to Remain in Place)</u>. For a street with AADT ≤50, an HS-10 loading is acceptable.
- (14) Width (Existing Bridge to Remain in Place). If the width of the existing bridge is less than the approach travelway width, strong consideration should be given to widening the bridge to at least the travelway width.
- (15) <u>Vertical Clearance (Local Under Railroad)</u>. Table values include an additional 150 mm allowance for a future pavement overlay. Vertical clearances apply from usable edge to usable edge of shoulder.
- (16) <u>Vertical Clearance (Existing Bridge)</u>. See Section 55-6.02 for additional information on minimum allowable vertical clearance.
- (17) <u>Vertical Clearance (Local Over Railroad)</u>. See Chapter Sixty-nine for additional information on railroad clearance under a highway.
- (18) <u>Intersection Sight Distance</u>. For left turn onto a 2-lane road, P = Passenger car; SU = single unit truck. See Figure 46-10G for values for combination trucks.